TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

BULLITT COUNTY BELLS MILL ROAD KY 1526 OVER FLOYDS FORK STA. 101+60.00

	ESTIMATE OF QUANTITIES																								
BID ITEM CODE	08100	08104	08150						03299	25028ED		23813EC	8301	21741NC	2585	2383	25078ED	2569	2726	26233EC	00001	00301	00221	02562	
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Steel Reinforcement	Steel Reinforcement, Epoxy Coated	PPC Box Beam SB42	Concrete Sealing	Masonry Coating	Remove Concrete Masonry	Armored Edge for Concrete	Rail System Single Slope 40 Inch	Structure Granular Backfill	Deck Drains	Remove Superstructure	Maintain and Control Traffic	Edge Key	Remove and Reset Guardrail	Thrie Beam Guardrail Connector TL3	Demobilization	Staking	Mobilization for Concrete Sealing	DGA Base ③	CL2 Asph Surf 0,38D PG64-22	CL2 Asph Base @ 0.75D PG64-22	Temporary Signs	
UNIT	C.Y.	C.Y.	LBS.	LBS.	L.F.	S.F.	S.Y.	C.Y.	L.F.	L.F.	C.Y.	EA.	LS	EA	LF	LF	EA	LS	LS	LS	TON	TON	TON	S.F.	
End Bent #1	9.2			2014			35	6.5			83														
Pier #1	24.2		4896				82	14.3																	
Pier #2	24.3		4845				83	16.1																	
ਹੋ End Bent #2	9.2			2014			35	6.6			83														
Pier #2 End Bent #2																									
SQ																									
วัไ																									
Superstructure		247.6		77663	1117.3	17076			50	626		10	1	1		100	4	1	1	1					<u> </u>
BRIDGE TOTALS	66.9	247.6	9741	81691	1117.3	17076	235	43.4	50	626	166	10	1	1	50	100	4	1	1	1	94	84	90	229	

- (1) Quantity is figured for 150 linear feet of 1.25" thick pavement overlay at each end of bridge.
- Quantity is figured for 25 linear feet of full depth roadway repleacement at each end of bridge, 2 lifts of 4⁴ and 4⁴.
- (3) Quantity is figured for 25 linear feet of 8° deep full depth roadway replacement at each end of bridge

	IND	EX OF SH	FTS	
01	1110			_
Sheet No.		Descripti	on	
S1	Title Sheet			_
S2	General No	tes		
S3	Layout			
S4	Removal De	etails		
S5-S6	End Bents			
S7	Pier 1			
S8	Pier 2			
S9	Framing Pla	an		
S10-S11	PPC Box B	eam SB42 Details		
S12-S13	Superstruct	ure		
S14-S15	Constructio	n Elevations		
S16	Intermediate	e Diaphragms		Т
S17	ECA and D			
S18	Detour Plan			_
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69 Emba		IAL PROV dge End Bent Struc		
69 Emba				
69 Emba	nkment at Bri	dge End Bent Struc	tures	
69 Emba	nkment at Bri	dge End Bent Struc	tures	
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BBP-003-	STANE	DARD DRA	wings	
BBP-003- BBP-002-	STANE 02 Elastome 04 Bearing D	DARD DRA	wings	
BBP-003- BBP-002- BGX-006-	STANE 02 Elastome 04 Bearing E 10 Stencils	DARD DRA Pric Bearing Pads for Details for Structures	WINGS Box Beams	
BBP-003- BBP-002- BGX-006- BHS-010	STANE 02 Elastome 04 Bearing I 10 Stencils i Railing S	DARD DRA Vici Bearing Pads for Details for Structures ystem 40 Inch Singl	WINGS Box Beams	
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BBP-003- BBP-002- BGX-006- BHS-010	STANE -02 Elastome -04 Bearing I -10 Stencils i Railing S -04 Bridge D	DARD DRA Vici Bearing Pads for Details for Structures ystem 40 Inch Singler	WINGS Box Beams	
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SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction with current Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN LOAD: This bridge is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by

FUTURE WEARING SURFACE: This structure is designed for a 15 PSF future wearing surface load.

DESIGN STRESSES: Concrete Class "A" \sim f'c = 3500 psi

Concrete Class "AA" f'c = 4000 psiSteel Reinforcement Fy = 60,000 psiStructural Steel Yield Strength Fv = 50,000 psi

DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specifications.

WIND LOAD: This bridge is designed for a wind load based on a wind velocity of 100 mph.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Any reinforcement bars designed be suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SHOP DRAWINGS: Submit shop drawings that are required by the plans and specifications directly to the Division of Structural Design. Is any changes in the design plans are proposed by a fabricator or supplier, submit those changes to the Department through the Contractor.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions

SUPERSTRUCTURE SLAB: Ensure the entire superstructure slab is poured continuously, out to out, before allowing any concrete to set.

MASONRY COATING: Apply masonry coating to substructures according to the Specifications. Do not apply masonry coating where Concrete Sealer is called out in these plans on the superstructure.

CONCRETE SEALER: The superstructure deck, barriers and overhangs shall also be sealed as shown herein these plans. Concrete surfaces (except the deck) shall receive the ordinary surface finish as described in section 601.03.18(A) prior to being sealed.

CONCRETE: Class "AA" is to be used throughout the new superstructure. Class "A" is to be used on the End Bents and Piers

ORIGINAL DRAWING NUMBER: Refer to Drawing Number 17586 for original plans.

GENERAL NOTES

ON-SITE INSPECTION: Each contractor submitting a bid for this work shall make a thorough inspection of the project site prior to submitting a bid and shall be thoroughly familiarized with existing conditions so that work can be expeditiously performed after a contract is awarded. Submission of a bid will be considered evidence of this inspection having been made. Any claims resulting from site conditions will be be honored be the Department of Highways.

DAMAGE TO THE SUBSTRUCTURE: The contractor is responsible for any and all damages to the existing substructures during reconstruction even to the replacement of the entire substructure, should they be damaged due to their actions.

CONCRETE REMOVAL: The pier columns and other concrete where the existing reinforcement is to be reused, the contractor shall use hand held jack hammers or hydro-demolition techniques to remove concrete without damaging the existing reinforcement that is to remain in place. Any concrete removal outside the detailed limits shall be replaced at the contractor's expense. The contractor shall make a saw cut at the removal limits to form a neat construction joint. All costs of this procedure are included in the price bid for "Remove Concrete Masonry".

DRILLING AND GROUTING: In accordance with Section 826 of the specifications, drill holes to a depth as shown herein these plans and apply a Type IV epoxy bonding adhesive in the holes. Also apply a Type V epoxy bonding material to the interface between the existing concrete and the new concrete prior to placing the new concrete. All costs associated with this work shall be incidental to the unit price bid for Class "A" Concrete.

EXISTING REINFORCING STEEL: The costs of cutting, bending and cleaning existing reinforcing steel is to be incidental to the lump sum bid for "Remove Superstructure".

REMOVE SUPERSTRUCTURE: Include in the lump sum bid for "Remove Superstructure" all costs (materials, labor, equipment, etc.) associated with removing and disposing of the existing superstructure as detailed herein in accordance with Section 203 of the Specifications. Also include in this lump sum bid the cost of any required excavation and subsequent backfilling (including materials, labor, equipment, etc.) behind the end bents. The cost of removing portions of the end bents and piers shall be included in the unit price bid for "Remove Concrete Masonry".

DIMENSIONS AND ELEVATIONS: All dimensions and elevations given in these plans are based on field surveyed data and dimensions from the old plans. Prior to beginning work or ordering any materials, the contractor shall verify all dimensions and elevations. No claim shall be honored by KYTC regarding site conditions.

EXISTING HANDRAIL: Remove and relocate the existing aluminum handrail as directed by the Engineer. All costs to remove, deliver to a location as specified by the Engineer, or disposal fees shall be incidental to the lump sum for "Remove Superstructure".

STRUCTURE GRANULAR BACKFILL: Excavation into existing pavement or ground behind end bent that may be required for end bent construction shall be backfilled with Structure Granular Backfill in accordance with Special Provision 69. Wrap all rock in Geotextile Fabric Class 2. All geotextile fabric shall be incidental to the unit price bid for "Structure Granular Backfill",

MASTIC TAPE: Mastic tape application is required at the end bents as shown in the Joint Waterproofing Detail on sheet S13. See sheet S13 for all mastic tape requirements. The cost of labor, materials, and incidental items for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for Concrete Class "AA" and no separate measurement or payment shall be made

MAINTAIN AND CONTROL TRAFFIC: Contractor will be responsible for all traffic control, signs, detours, type 3 barriers, etc. All costs shall be incidental to maintain and control traffic.

PAVEMENT: The area in the estimate of quantities for pavement includes all areas on the approaches shown in the plans. The contractor shall provide a minimum 8" of DGA, two 4" lifts of asphalt base, and a minimum of $1\frac{1}{4}$ " asphalt surface. The price bid for the DGA and pavement quantities includes all materials, labor, and equipment necessary to place full depth pavement where necessary, and an overlay where the existing pavement structure is not removed. Construction shall be done in accordance with the plans, specifications, and as the Engineer directs. Begin overlay 150' before begin bridge station and extend to 150' after end bridge station, not including bridge deck.

The following abbreviations may have been used in the preparation of these plans:

between Back Face b.f. BOF Bottom of Footing BOS Bottom of Slab

bot. Bottom Brg. Bearing

C to C Center to Center Current Edition c.e. C.Y. Cubic Yards Chd. Chord CL Center Line Clr. Clear Conc. Concrete

CubicCu. DrawingDwg

Fach Face e.f. EI. Elevation eq. Egual Estimate Est.

ExteriorExt. Face to Face F to F f.f. Front Face f.s. Far Side Front Inside Diameter I.D.

Inch in. Int Interior

LBS Low Bridge Seat LBS. Pounds Meter МРН Miles Per Hour

Near Side n.s. Outside Diameter O.D.

Opp. Opposite PC Point of Curvature

Perp. Perpendicular Point of Intersection

PPC Precast Prestressed Concrete PPCDU Precast Prestressed Deck Unit PSI Pounds per Square Inch

РΤ Point of Tangency Radius

Riaht

RCBC Reinforced Concrete Box Culvert RCDG Reinforced Concrete Deck Girder

Reg'd Required RR Railroad Shld Shoulder spa. Spaces Sta. Station Std. Standard Str. Straight Tan Tangent Thru Through TOF Top of Footing

TOS Top of Slab Tot. Total Тур. Typical Vert Vertical W.P. Working Point Yard

REVISION COMMONWEALTH OF KENTUCKY KENTUCKY DEPARTMENT OF HIGHWAYS

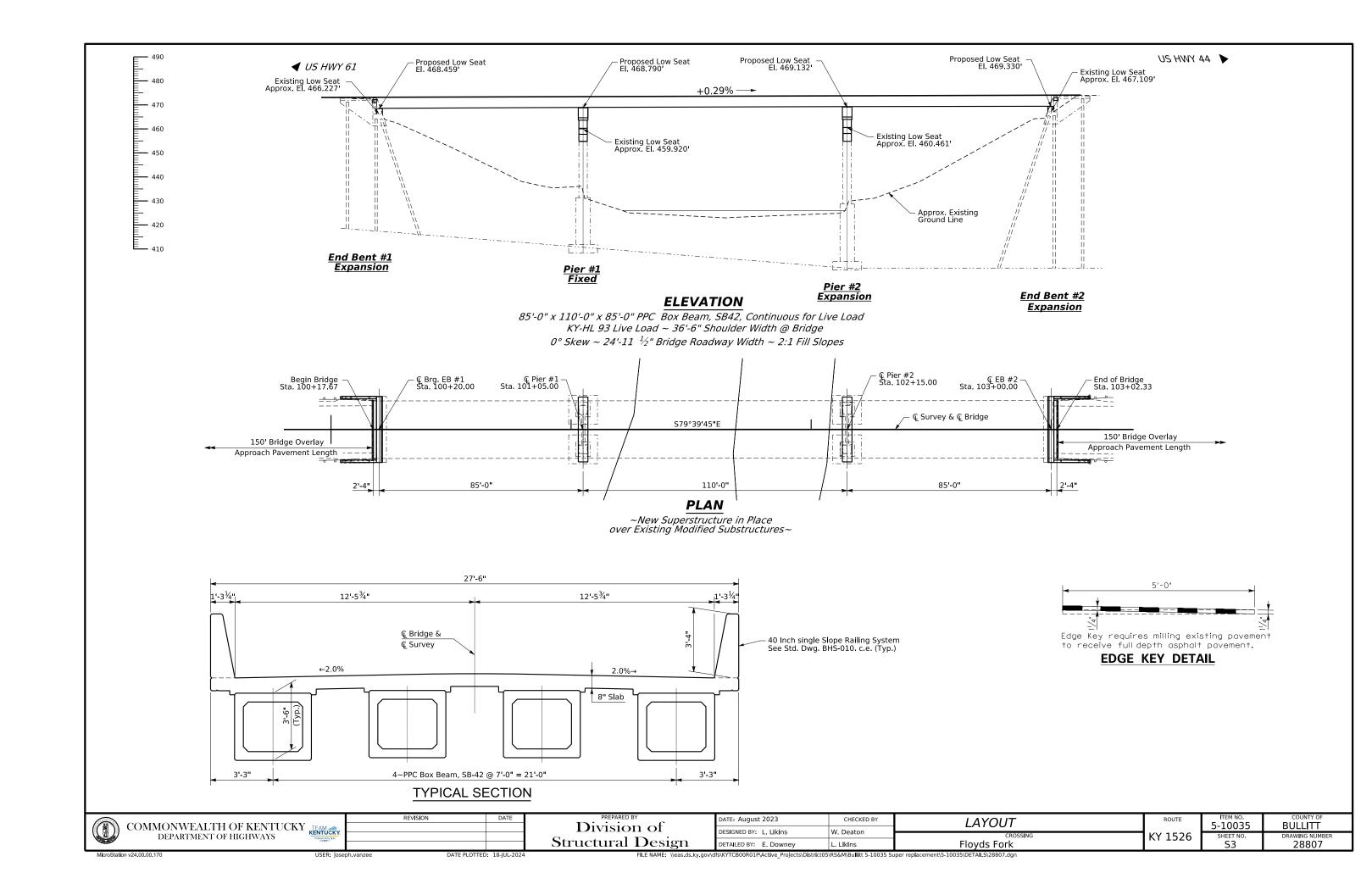
Structural Design

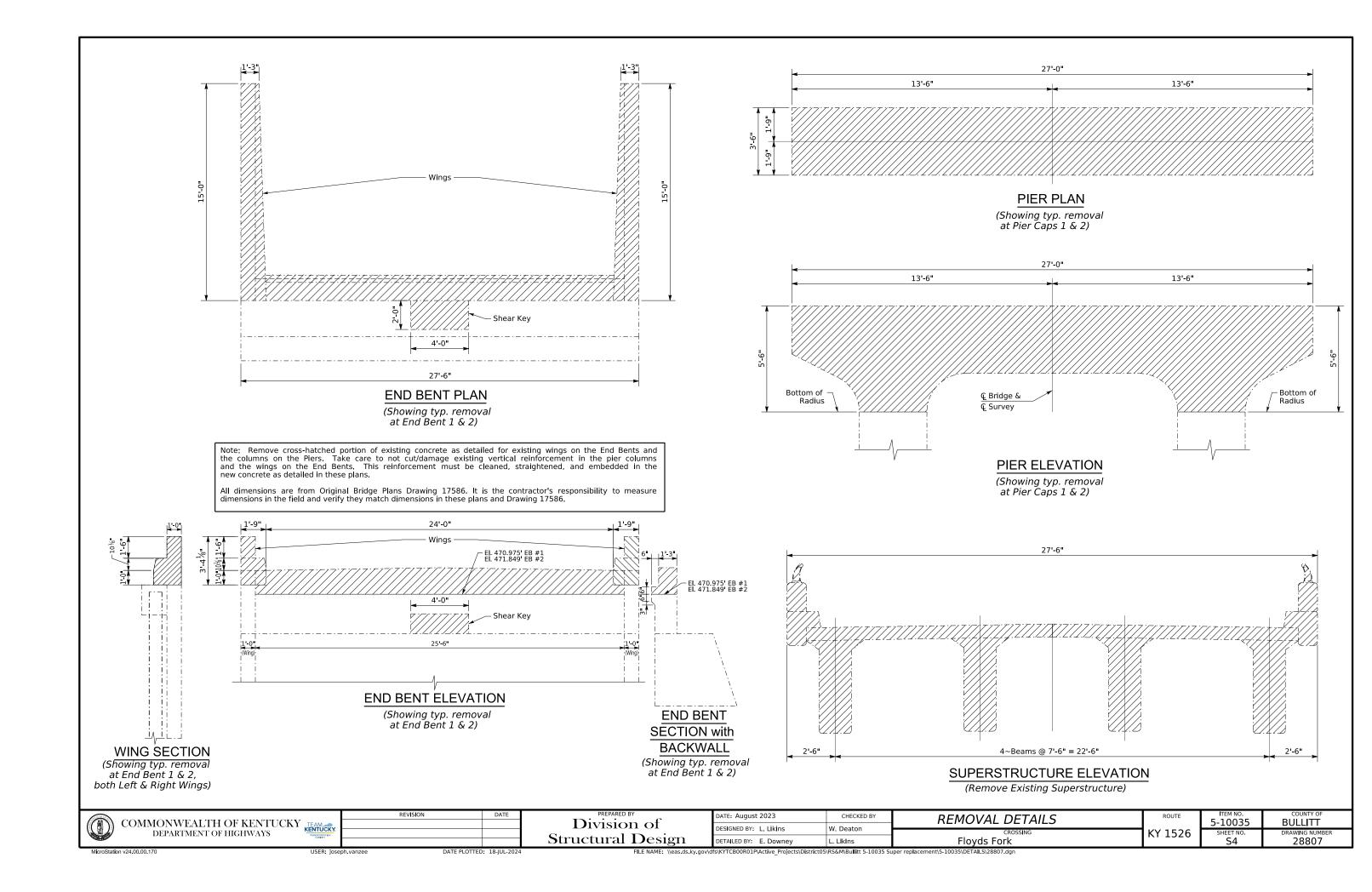
Division of

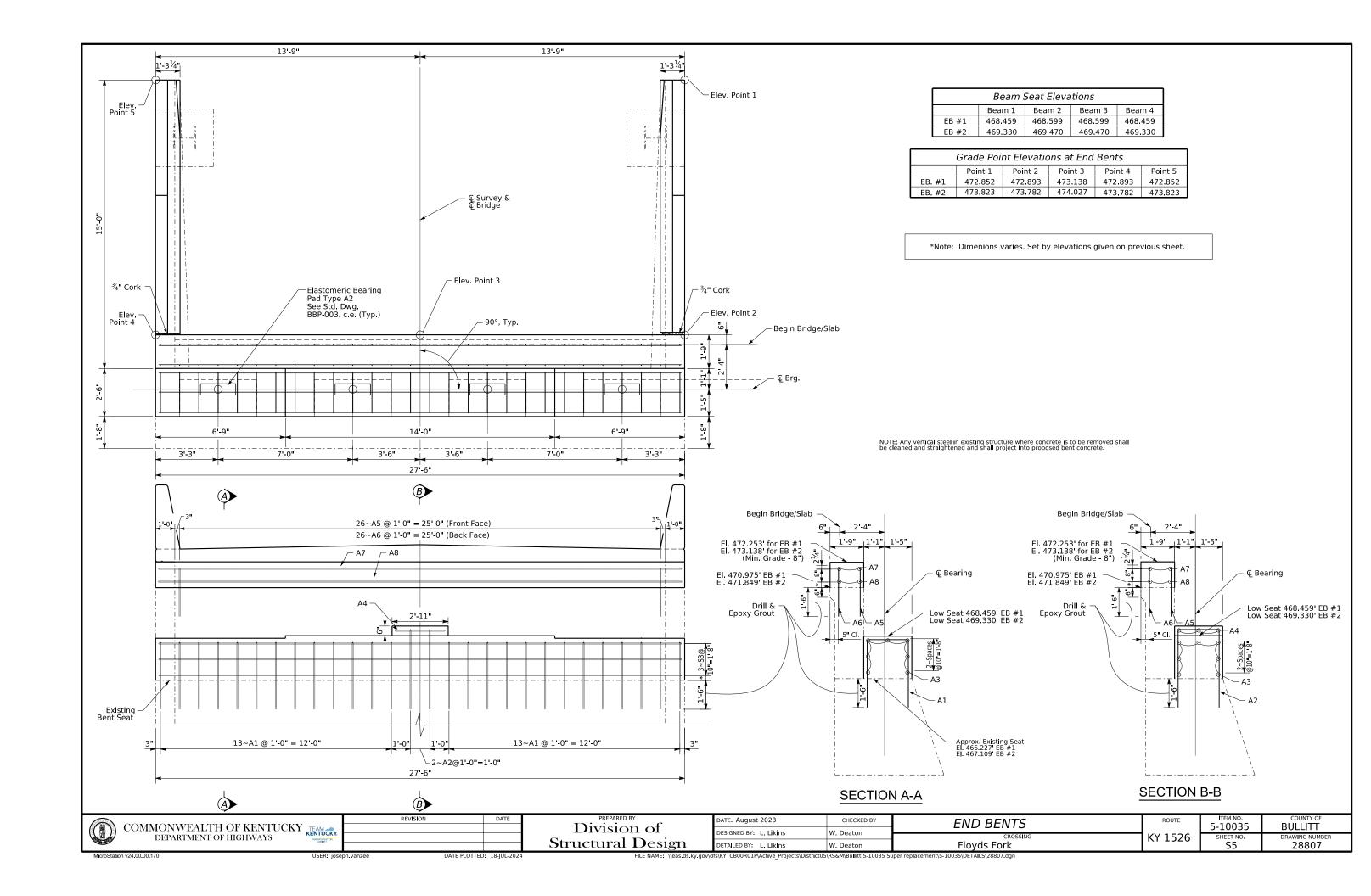
DATE: August 2023 CHECKED BY DESIGNED BY: L. Likins W. Deaton DETAILED BY: E. Downey L. LIkins

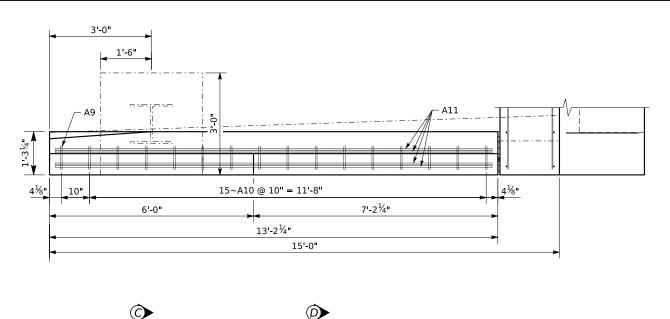
GENERAL NOTES Flovds Fork

5-10035 BULLITT KY 1526 28807









NOTE: Any vertical steel in existing structure where concrete is to be removed shall be cleaned and straightened and shall project into proposed bent concrete.

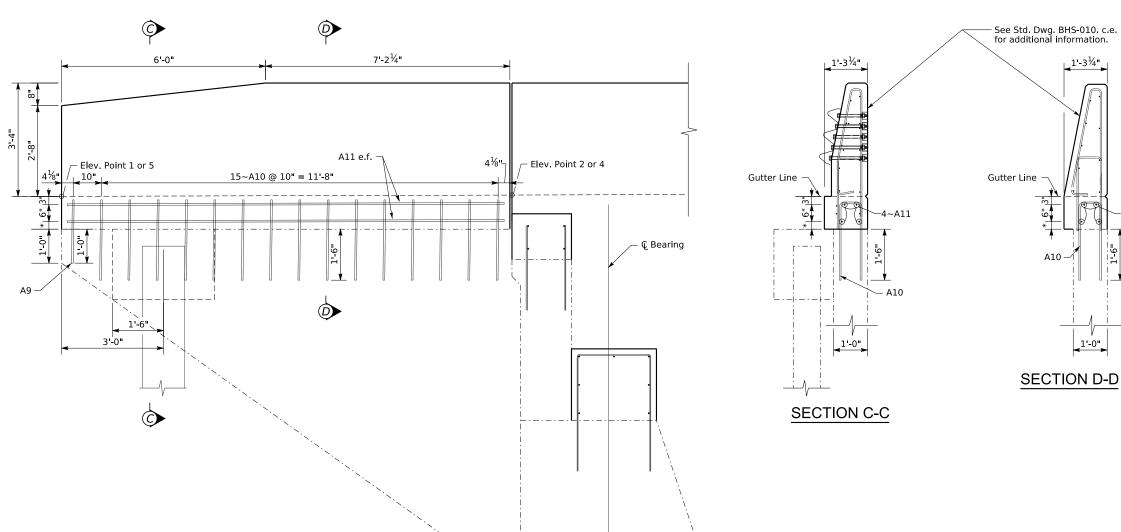
			BIL	L OF	REINFORCEMENT		
MARK	TYPE	NO.	SIZE LENGTH		LOCATION	Α	В
A1e	2s	52	5	9-1	Cap Stirrup	3-6 %	2- 2
A2e	2s	4	5	10-5	Cap Stirrup	4-2 %	2- 2
A3e	Str.	14	5	27- 2	Cap Horizontal		
A4e	Str.	6	5	2- 7	Top of Shear Key		
A5e	Str.	52	5	2- 6	Backwall Front Face Vertical		
A6e	Str.	52	5	2- 6	Backwall Back Face Vertical		
A7e	Str.	4	5	27- 2	Backwall Top Horizontal		
A8e	Str.	4	5	27- 2	Backwall Horizontal		
A9e	2s	4	5	4-2	Wing Stirrup	1-10 %	0-8
A10e	2s	60	5	5-2	Wing Stirrup	2-4 3/8	0-8
Alle	Str.	16	5	12-10	Wing Horizontal		

TYPE 2

*Note: Dimenions may vary. Set by elevations given on previous sheet.

1'-31/4"

1'-0"



COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

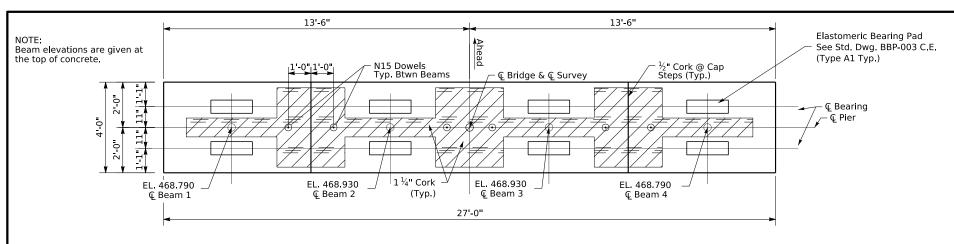
REVISION DATE PLOTTED: 18-JUL-2024

WING ELEVATION

Division of Structural Design

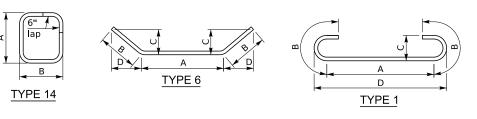
DATE: August 2023 CHECKED BY **END BENTS** DESIGNED BY: L. Likins W. Deaton DETAILED BY: L. LIKINS W. Deaton Floyds Fork

5-10035 BULLITT KY 1526 RAWING NUMB 28807



BILL OF REINFORCEMENT MARK TYPE NO. SIZE LOCATION С N1 Column Vertical Str. 32 13-6 N2 14s 36 13-3 Column Ties N3 10 26-11 Cap Bottom Bars 20-3 1/8 | 3-3 1/4 | 0-10 1/8 | 3-2 1/4 12 27- 2 Cap Side Str. 22-6 Cap Sides 26- 0 ³/₄ 2- 2 3-10 ³/₈ 2- 6 ⁵/₈ 4- 0 2- 6 ⁵/₈ 10 30- 5 Cap Top Bars 1-1 1/4 13-4 14s 5 Cap Stirrup N8 14s 13- 7 4 Cap Stirrup N9 14s 13-10 Cap Stirrup 4-1 % 2-6 % N10 14s 4 14- 2 4-3 % 2-6 % Cap Stirrup N11 14s 14- 5 Cap Stirrup 4-5 2-6 % N12 14s 4 5 14-8 Cap Stirrup 4-6 3/4 2-6 5/8 N13 14s 4 14-11 Cap Stirrup 4-8 2-6 % N14 14s 46 14-11 Cap Stirrup 4-8 2-6 % 2- 0 Cap Dowels

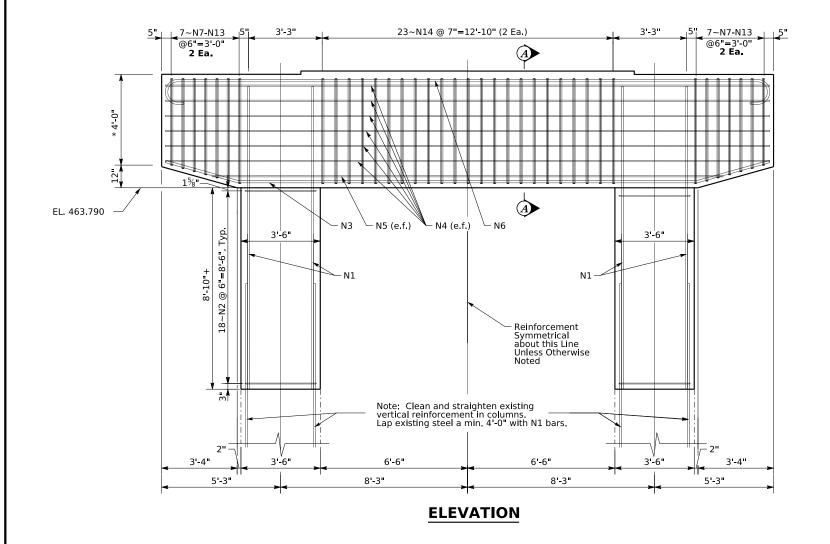
* 1 ½" Diameter Dowels may be commercial grade steel

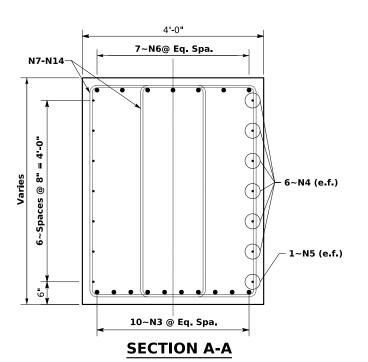


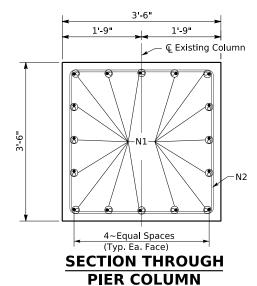
Note: For bearing and dowel details, see Std. Dwg. BBP-002, c.e.



Note: See removal details sheet for details regarding removal of existing concrete.







BULLITT

28807

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

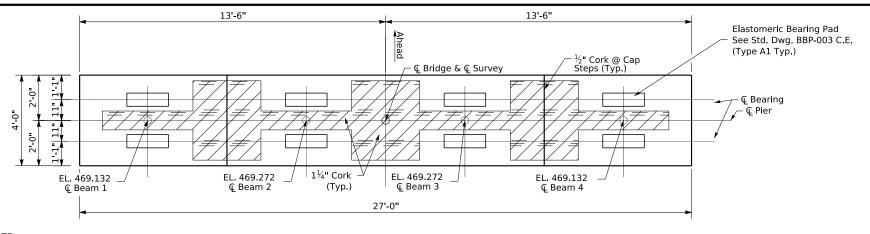
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Division of Structural Design

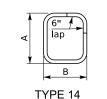
DATE: August 2023 CHECKED BY DESIGNED BY: L. Likins W. Deaton DETAILED BY: L. LIKINS W. Deaton

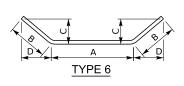
PIER 1 5-10035 KY 1526 Floyds Fork

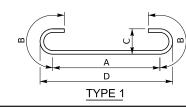
* Measured @ Low Bridge Seat







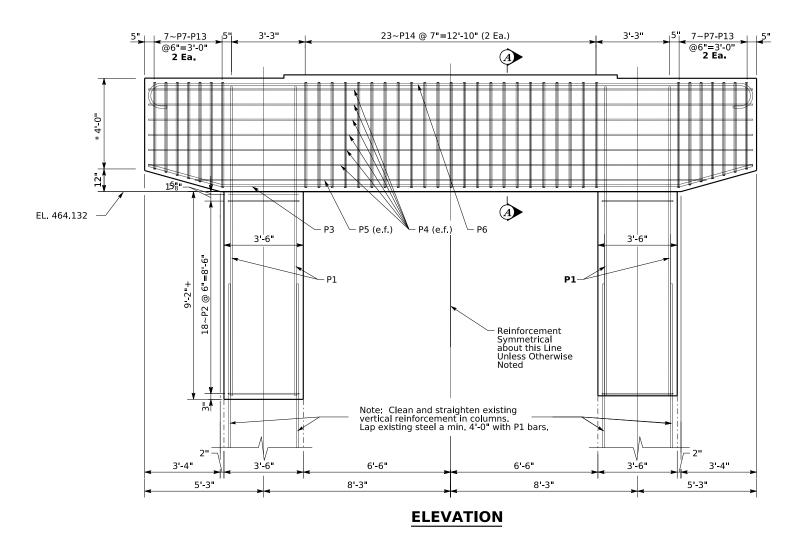


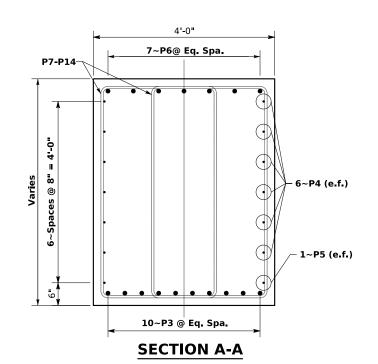


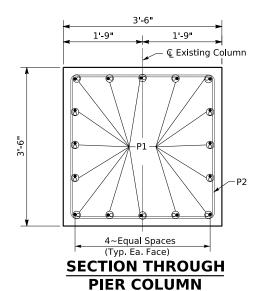
NOTE: Beam elevations are given at the top of concrete.

* Measured @ Low Bridge Seat

PLAN OF CAP







Note: See removal details sheet for details regarding removal of existing concrete.

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

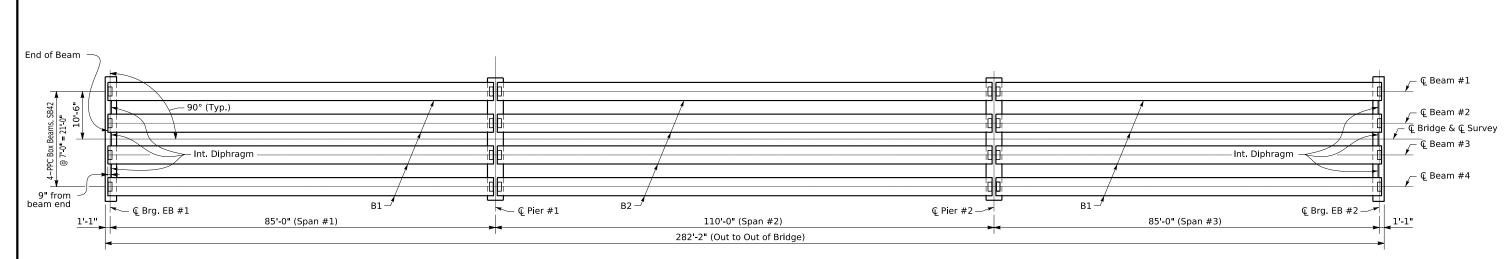
REVISION

Division of Structural Design

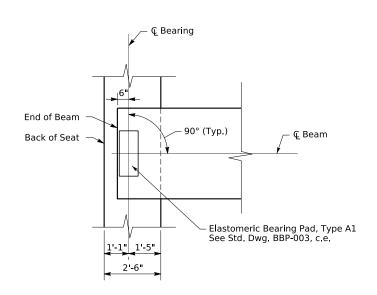
DATE: August 2023 CHECKED BY DESIGNED BY: L. Likins W. Deaton DETAILED BY: L. LIKINS W. Deaton

PIER 2 Floyds Fork

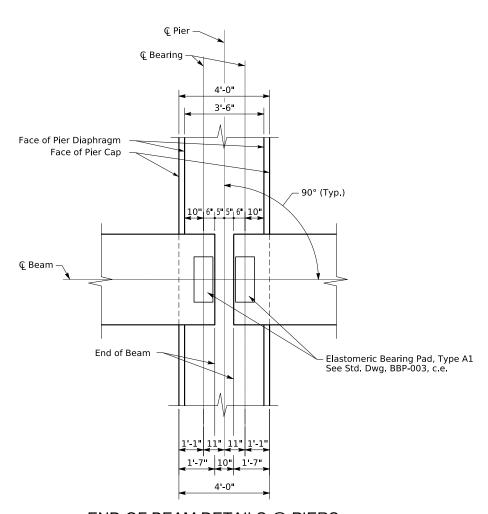
5-10035 BULLITT KY 1526 28807



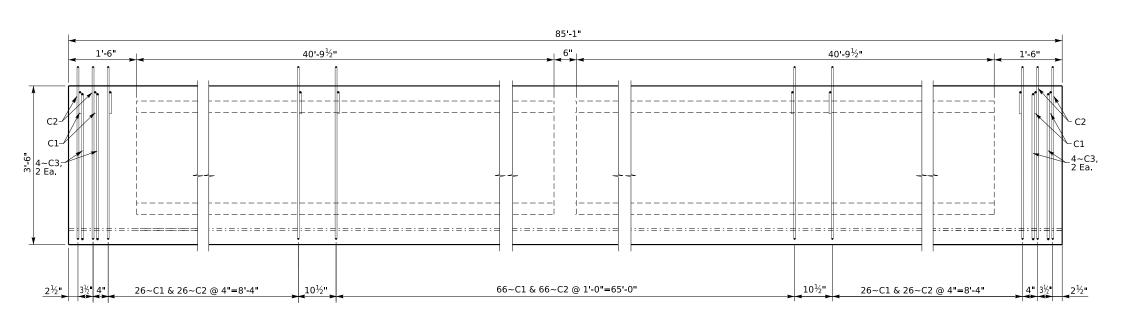
FRAMING PLAN



END OF BEAM DETAILS @ END BENTS

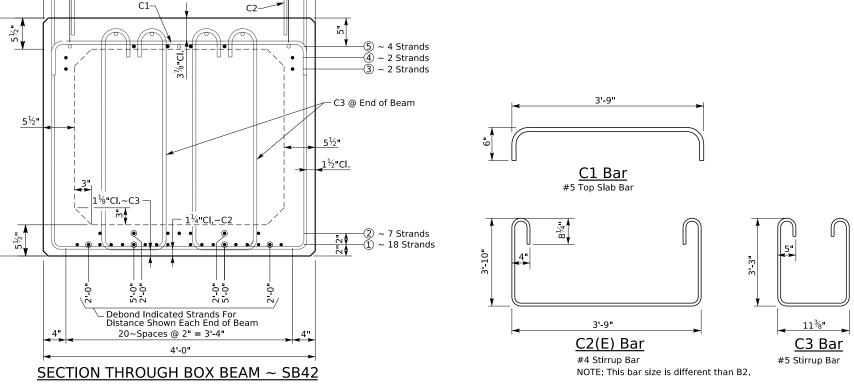


END OF BEAM DETAILS @ PIERS



ELEVATION - Showing Dimensions and Reinforcement

(Measured along © Beam)



Bar		

Void Drain Two drains are to be located at each end of each void.

Drains shall be 1"Ø of a type approved by the Department's Division of Materials. 12"

Section C-C

10" / ¾" Rustication Groove

Cable Clamps

VOID DRAIN DETAIL

Note: A1 Bars~2 Lengths, 2'-2" Min. Lap C1 and C3 bars are #5. C2 bars are #4 REVISION COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

End

1 2 3 4 5

18 7 2 2 4

Fully Stressed

9,000 10,000

Division of Structural Design DETAILED BY: B. MIller

DATE: August 2023 CHECKED BY PPC BOX BEAM SB42 DETAILS DESIGNED BY: L. Likins W. Deaton L. LIkins Floyds Fork

End of

5-10035 BULLITT KY 1526 28807

General Notes

CONCRETE: Ensure prestressed girder concrete is in accordance with

PRESTRESSING REINFORCEMENT: Ensure that strands are 0.6" (nominal diameter, 0.217 sq. in.), uncoated seven-wire stress relieved, low-relaxation conforming to AASHTO M 203, Grade 270. Billing of the cost for redesign of beam and subsequent plan modifications will be made for any request of alternate strand

type or arrangement. The designer of the original plans is

CONSTRUCTION METHOD: Pretension all beams. Ensure concrete has attained f'ci (shown in the table) in standard test cylinders that are made and cured identically with the beams without bond stresses being transferred to the concrete or releasing the end anchors. Attain f'c (shown in the table) at or prior to 28 days. Apply an initial force of 43,943 lbs. per

low-relaxation strand to develop a stress of 202,500 psi. No beam will be accepted that is honeycombed to the extent that strength of the beam or resistance to deterioration has been affected. An allowance of 0.0005L is made for shortening of beams due to shrinkage and elastic change. Show a detensioning plan by sequential numbering of the strand

LIFTING DEVICES: Detail lifting devices on the shop plans. Loads are to be

BEARING DEVICES: Include the price for lead plates and/or bearing pads and any necessary galvanized metal shims in the bid for precast beams.

responsible for any necessary adjustments to assure a minimum slab thickness of eight (8) inches as shown in the plans. This work will be

considered incidental to the completion of the structure and have the

considered a stirrup for purposes of bend diameters.

10",

FABRICATION: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the contractor will be

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. Epoxy Coated Reinforcement shall be in accordance with Section 811.10 of the Specifications. All bars marked "C" shall be

Strand Splicing Detail

MATERIALS DESIGN SPECIFICATIONS: For prestressed beams:

these plans and the specifications.

FY = 60,000 psi F'S = 270,000 psi

responsible for the billing and work.

pattern on the shop plans.

approval of the Engineer

End of

Strands

heating Elastomeric Bearing Pad, see Std. Dwg. BBP-003, c.e

extended 1'-5" and bent without

Cable Clamps

distributed equally to each device.

Strand Data with number indicated in rows

7~A1 Spaced as Shown

78,613 122 122 8

Midspan

1 2 3 4 5

B1 18 7 2 2 4

Fully Stressed

Tota

of

Strands

33

DATE PLOTTED: 18-IUL-2024

Box Beam Data

Concrete Stress (psi) # of (psi) Beams (lbs) (C Bars (lbs) C1 [C2 [C3

Maximum

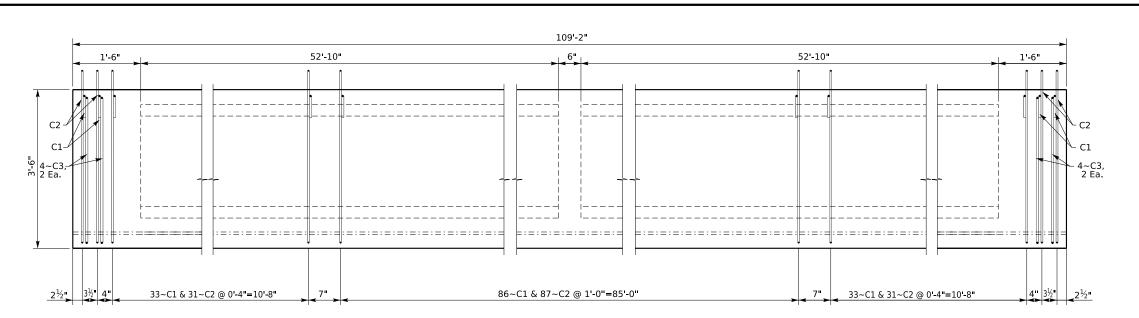
Camber

Straight

Mark Size Length

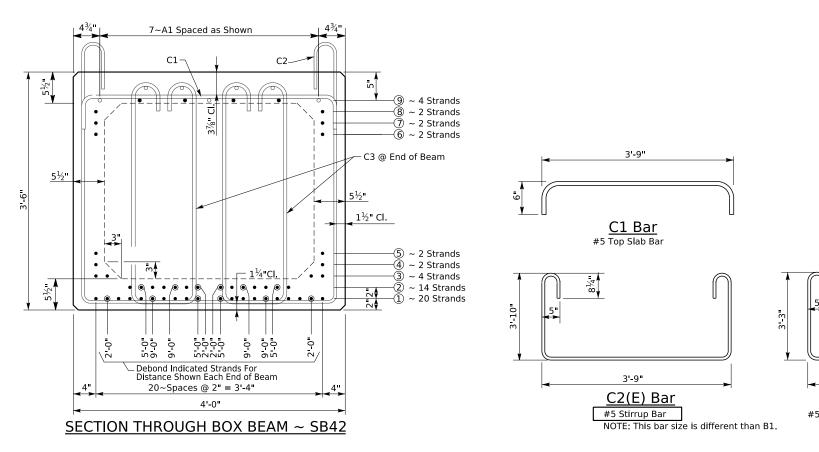
A1 #5 43'-6"

Reinforcement Allowable



ELEVATION - Showing Dimensions and Reinforcement

(Measured along & Beam)





REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. Epoxy Coated Reinforcement shall be in accordance with Section 811.10 of the Specifications. All bars marked "C" shall be End of considered a stirrup for purposes of bend diameters. 10" / ¾" Rustication Groove Fnd of Strands extended 1'-5" and bent without 10", heating Elastomeric Bearing Pad, see Std. Dwg. BBP-003, c.e Cable Clamps Cable Clamps Strand Splicing Detail Section C-C ~Typical at Pier~ Void Drain

> Two drains are to be located at each end of each void.
>
> Drains shall be 1"Ø of a type approved by the Department's Division of Materials.

VOID DRAIN DETAIL

12"

Mark

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

Midspan

Fully Stressed

Strand Data with number indicated in rows

1 2 3 4 5 6 7 8 9 1 2 3 4 5 6 7 8 9

REVISION

of

Division of Structural Design

No. of

C1 | C2 | C3

Weight C Bars

Straight

Mark Size Lengt

A1 #5 55'-6"

Reinforcement Allowable

Maximum

Camber

Note: A1 Bars~2 Lengths, 2'-2" Min. Lap. All C bars are #5 for B2

ATE: August 2023 CHECKED BY DESIGNED BY: L. Likins W. Deaton DETAILED BY: B. MIller L. LIkins

PPC BOX BEAM SB42 DETAILS Floyds Fork

5-10035 BULLITT KY 1526 28807

General Notes

CONCRETE: Ensure prestressed girder concrete is in accordance with

PRESTRESSING REINFORCEMENT: Ensure that strands are 0.6" (nominal diameter, 0.217 sq. in.), uncoated seven-wire stress relieved, low-relaxation conforming to AASHTO M 203, Grade 270. Billing

of the cost for redesign of beam and subsequent plan modifications will be made for any request of alternate strand type or arrangement. The designer of the original plans is

CONSTRUCTION METHOD: Pretension all beams. Ensure concrete has attained f'ci (shown in the table) in standard test cylinders that are made and cured identically with the beams without bond stresses being transferred to the concrete or releasing the end anchors. Attain f'c (shown in the

table) at or prior to 28 days. Apply an initial force of 43,943 lbs. per low-relaxation strand to develop a stress of 202,500 psi. No beam will be accepted that is honeycombed to the extent that strength of the beam or resistance to deterioration has been affected. An allowance of 0.0005L is made for shortening of beams due to shrinkage and elastic change. Show a detensioning plan by sequential numbering of the strand

LIFTING DEVICES: Detail lifting devices on the shop plans. Loads are to be

BEARING DEVICES: Include the price for lead plates and/or bearing pads and any necessary galvanized metal shims in the bid for precast beams.

responsible for any necessary adjustments to assure a minimum slab thickness of eight (8) inches as shown in the plans. This work will be considered incidental to the completion of the structure and have the

FABRICATION: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the contractor will be

MATERIALS DESIGN SPECIFICATIONS: For prestressed beams:

these plans and the specifications.

FY = 60,000 psi F'S = 270,000 psi

responsible for the billing and work.

pattern on the shop plans.

approval of the Engineer

distributed equally to each device.

Box Beam Data

of

-|Beams|

Concrete Stress | Total | Approx.

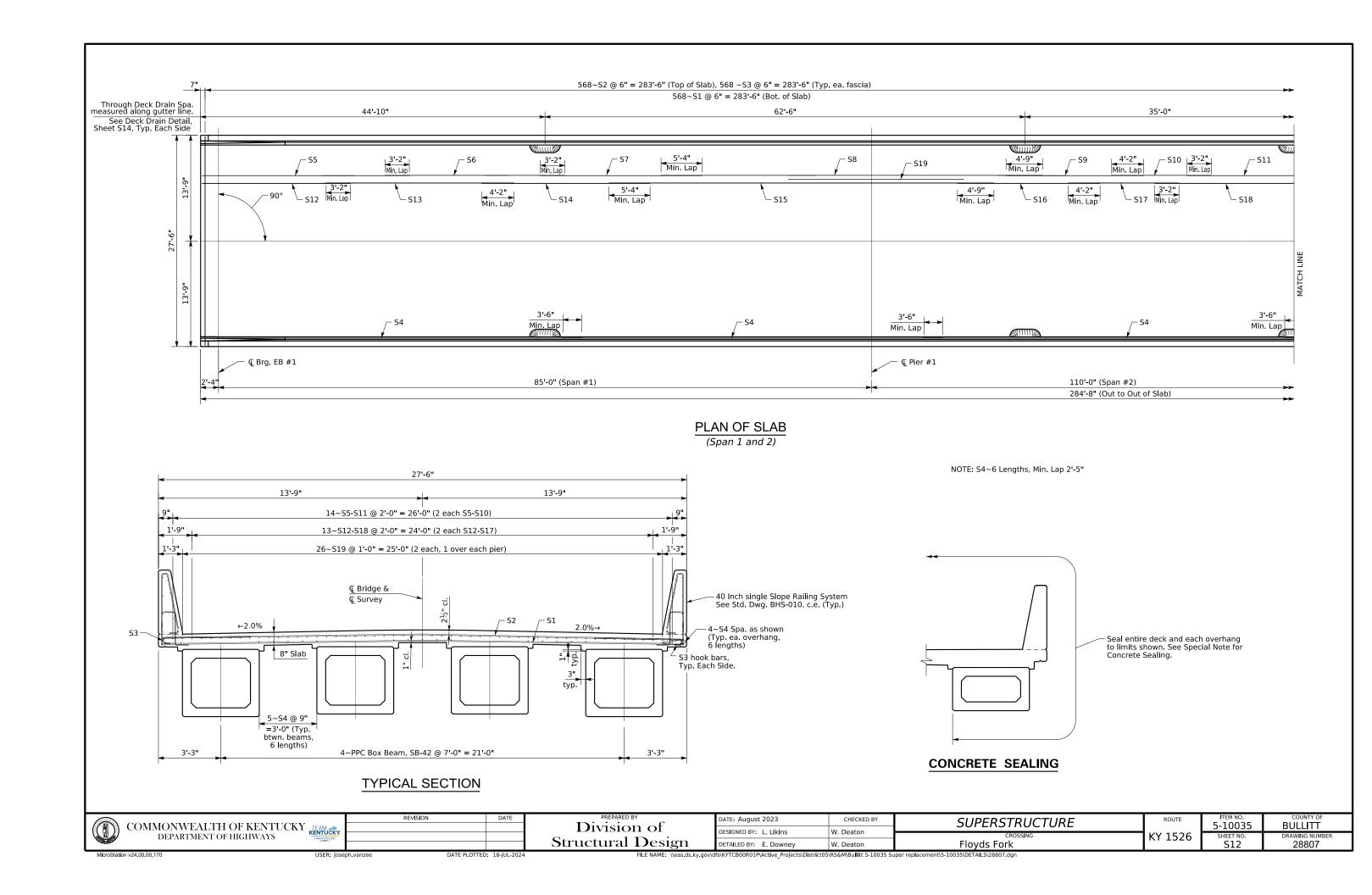
(psi)

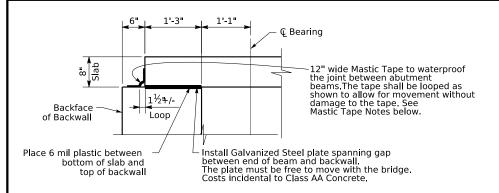
Strands f'ci f'c

End

Fully Stressed

B2 20 14 4 2 2 2 2 2 4 20 14 4 2 2 2 2 4 52 9,000 10,000 4





SLAB OVER BACKWALL DETAIL

(measured perpendicular to backwall)

MASTIC TAPE NOTES

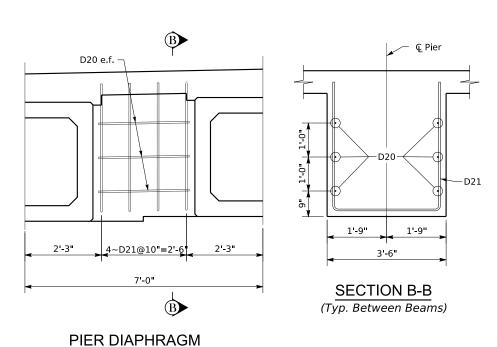
MASTIC TAPE: Mastic Tape used to seal joints is to meet the requirements of ASTM C-877 Type I, II, or III. The joint is to be covered with 12-inch wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape mfgr., shall be applied for a minimum width of nine inches on each side of the

Mastic Tape shall be either:

EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC., CADILLOC by the UP RUBBER CO. INC.

Mastic Tape shall cover the joint continuously unless otherwise shown in the plans. Mastic Tape shall be spliced by lapping a minimum of six inches and in accordance with the mfgrs. recommendations with the overlap running downhill.

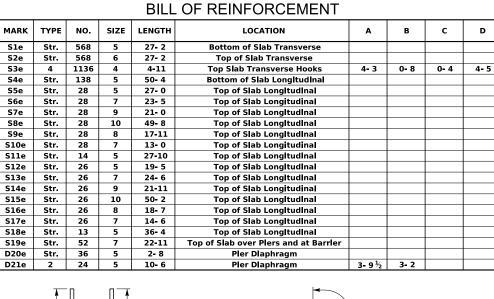
The cost of labor, materials, and incidental items for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for Concrete Class "AA" and no separate measurement or payment shall be made.

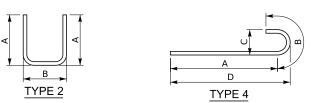


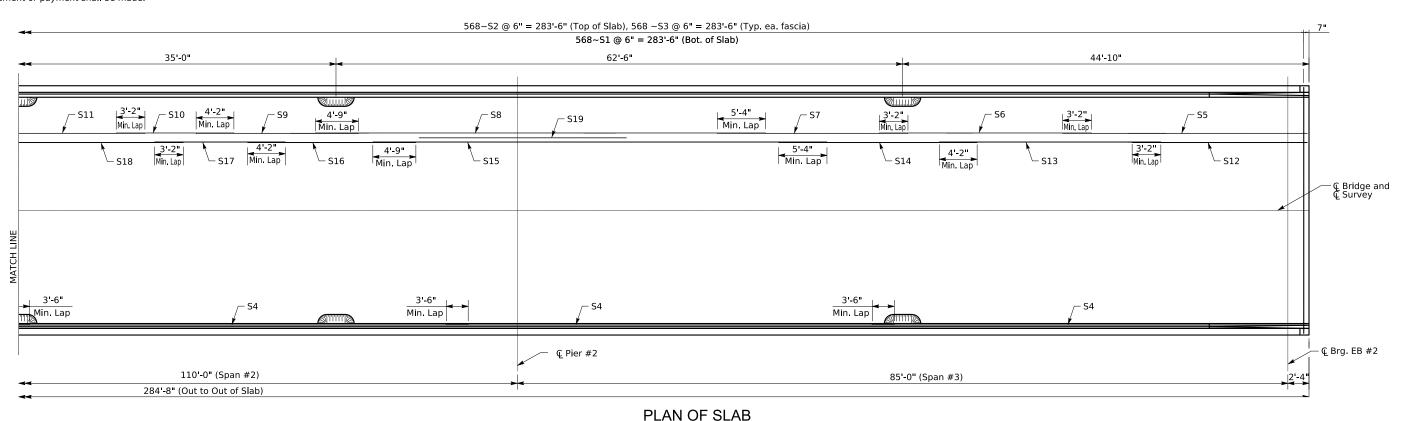
(Typ. Between Beams)

Division of

Structural Design







(Span 2 and 3)

DATE: July 2023

DESIGNED BY: L. Likins

DETAILED BY: E. Downey

COMMONWEALTH OF KENTUCKY

DEPARTMENT OF HIGHWAYS

REVISION

CHECKED BY

W. Deaton

L. Likins

SUPERSTRUCTURE

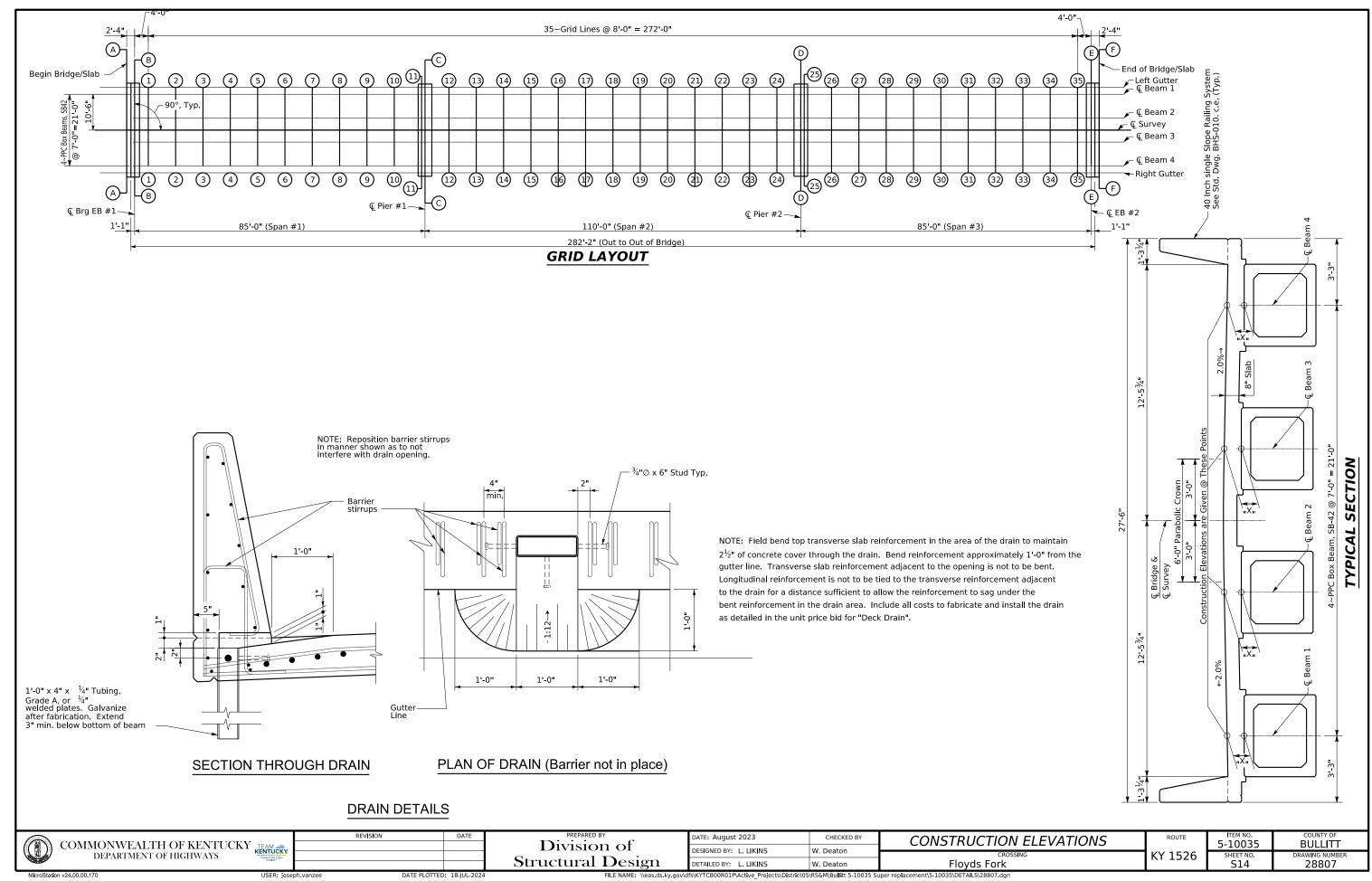
5-10035

SHEET NO

KY 1526

BULLITT

28807



CONSTRUCTION ELEVATIONS															
	LEFT		; BEAM 1			i BEAM 2				BEAM 3			i BEAM 4		RIGHT
LOCATION	GÜTTER	CONSTR. ELEV.	TOP OF BEAM	DIM. "X"	CONSTR. ELEV.	TOP OF BEAM	DIM. "X"	i	CONSTR. ELEV.	TOP OF BEAM	DIM.	CONSTR. ELEV.	TOP OF BEAM	DIM. "X"	GUTTER
Skew Line AA	472.920	472.960			473.100			473.140	473.100			472.960			472.920
Skew Line BB	472.927	472.967			473.107			473.147	473.107			472.967			472.927
Skew Line CC	473.192	473.231			473.371			473.411	473.371			473.231			473.192
Skew Line DD	473.534	473.573			473.713			473.753	473.713			473.573			473.534
Skew Line EE	473.798	473.837			473.977			474.017	473.977			473.837			473.798
Skew Line FF	473.805	473.845			473.985			474.025	473.985			473.845			473.805
Grld Line 1	472.952	472.992			473.132			473.172	473.132			472.992			472.952
Grid Line 2	473.001	473.041			473.181			473.221	473.181			473.041			473.001
Grid Line 3	473.047	473.087			473.227			473.267	473.227			473.087			473.047
Grid Line 4	473.087	473.127			473.267			473.307	473.267			473.127			473.087
Grld Line 5	473.121	473.161			473.301			473.341	473.301			473.161			473.121
Grid Line 6	473.149	473.188			473.328			473.368	473.328			473.188			473.149
Grid Line 7	473.169	473.208			473.348			473.388	473.348			473.208			473.169
Grid Line 8	473.182	473.221			473.361			473.401	473.361			473.221			473.182
Grld Line 9	473.189	473.229			473.369			473.409	473.369			473.229			473.189
Grid Line 10	473.192	473.231			473.371			473.411	473.371			473.231			473.192
Grid Line 11	473.192	473.231			473.371			473.411	473.371			473.231			473.192
Grid Line 12	473.260	473.300			473.440			473.480	473.440			473.300			473.260
Grid Line 13	473.336	473.376			473.516			473.556	473.516			473.376			473.336
Grid Line 14	473.406	473.446			473.586			473.626	473.586			473.446			473.406
Grid Line 15	473.469	473.509			473.649			473.689	473.649			473.509			473.469
Grid Line 16	473.522	473.562			473.702			473.742	473.702			473.562			473.522
Grid Line 17	473.564	473.604			473.744			473.784	473.744			473.604			473.564
Grid Line 18	473.595	473.635			473.775			473.815	473.775			473.635			473.595
Grid Line 19	473.614	473.654			473.794			473.834	473.794			473.654			473.614
Grid Line 20	473.621	473.661			473.801			473.841	473.801			473.661			473.621
Grid Line 21	473.618	473.658			473.798			473.838	473.798			473.658			473.618
Grid Line 22	473.605	473.645			473.785			473.825	473.785			473.645			473.605
Grid Line 23	473.585	473.624			473.764			473.804	473.764			473.624			473.585
Grid Line 24	473.559	473.598			473.738			473.778	473.738			473.598			473.559
Grid Line 25	473.540	473.579			473.719			473.759	473.719			473.579			473.540
Grid Line 26	473.590	473.629			473.769			473.809	473.769			473.629			473.590
Grid Line 27	473.637	473.676			473.816			473.856	473.816			473.676			473.637
Grid Line 28	473.679	473.719			473.859			473.899	473.859			473.719			473.679
Grid Line 29	473.716	473.755			473.895			473.935	473.895			473.755			473.716
Grid Line 30	473.746	473.785			473.925			473.965	473.925			473.785			473.746
Grid Line 31	473.768	473.808			473.948			473.988	473.948			473.808			473.768
Grid Line 32	473.784	473.823			473.963			474.003	473.963			473.823			473.784
Grid Line 33	473.793	473.833			473.973			474.013	473.973			473.833			473.793
Grid Line 34	473.797	473.837			473.977			474.017	473.977			473.837			473.797
Grid Line 35	473.798	473.838			473.978			474.018	473.978			473.838			473.798

NOTES FOR ELEVATIONS TAKEN ON PRESTRESSED CONCRETE BEAMS

Take elevations on top of beam at points indicated by the grid layout. The beam elevations are to be read to three decimals, and entered in tables under "Top of Beam" elevations.

Compute dimension "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals dimension "X". Construction Elevations include camber due to weight of the concrete slab and barrier. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations,

Construct barrier to roadway grade. Do not add camber to the barrier.

Note to Resident: The "Maximum Allowable Camber" shown on the beam sheet is the amount of camber, measured prior to casting the deck, above which the beam will begin to encroach into the slab. If the measured camber is greater than the "Maximum Allowable Camber" the contractor will be responsible for any necessary adjustments to assure a minimum slab thickness as shown in the plans. This work will be considered incidental to the completion of the structure and must have the approval of the Engineer.

The minimum allowable X-Dimension on a beam is that which results in the design deck thickness (8") at the edge of the beam flange. This is calculated as the deck thickness + (half the top flange width x cross slope of the bridge). For example 8" + 24"x0.02 = 8.48" = 0.706'. Any necessary modifications to some or all of the X-dimensions must meet approval of the Engineer.

Division of Structural Design

DATE: August 2023

CHECKED BY

DESIGNED BY: L. LIKINS

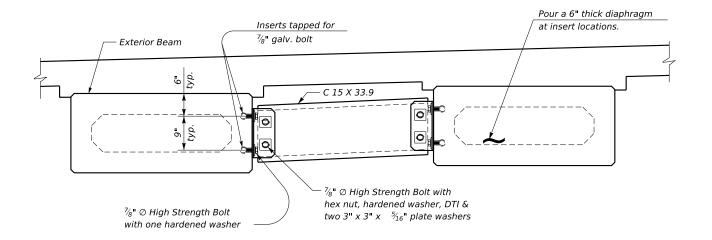
DETAILED BY: L. LIKINS

W. Deaton

CROSSING

Floyds Fork

| SHEET NO. | DRAWING NUMBER | S15 | 28807



INTERMEDIATE DIAPHRAGM

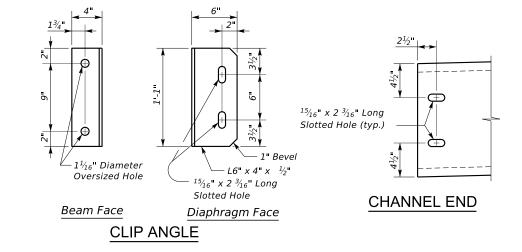
Diaphragm Notes

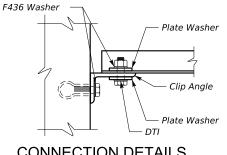
CONNECTIONS: Ensure all bolted connections are ASTM A325, \(\frac{7}{8} \) inch diameter high strength bolts, nuts, and washers, mechanically zinc coated in accordance with AASHTO M298, for Class 50. Install all high strength bolted field connections using "direct tension indicators" (DTI's) in accordance with the Standard Specifications and ASTM F959. Ensure all DTI's are mechanically zinc coated. Show installation details of the DTI's on the shop plans. Place DTI's under the bolt head.

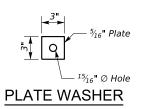
STRUCTURAL STEEL: Ensure plates, angles, and channels conform to ASTM A36 or A572 and galvanized after fabrication.

SHOP DRAWINGS: Show the location of all inserts and holes on the precast beam shop drawings. Submit shop drawings for the steel diaphragms to the Division of Structural Design for approval.

DIAPHRAGMS: Erect the diaphragms the same day that the precast beams are placed on the substructure. Include the cost of all materials and labor required to fabricate and erect the diaphragms in the bid for Precast Beams.







CONNECTION DETAILS

COMMONWEALTH OF KENTLICKY	REVISION	DATE	PREPARED BY	DATE: August 2023	CHECKED BY	INTERMEDIATE DIAPHRAGMS	ROUTE	ITEM NO.	COUNTY OF
I E SK YA COMINION WEALTH OF KENTUCKI TEAM 🗢 T			Division of	DESIGNED BY: C. Van Zee	L. Likins	INTENMEDIATE DIALTINAUMS		5-10035	BULLITT
DEPARTMENT OF HIGHWAYS KENTUCKY			0. 1.	DESIGNED BY: C. Van Zee	L. LIKINS	CROSSING	KY 1526	SHEET NO.	DRAWING NUMBER
CARRET			Structural Design	DETAILED BY: M. Bawlthawng	L. LIkins	Floyds Fork	1516	S16	28807

USER: joseph vanzee DATE PLOTTED: 18-JUL-2024 FILE NAME: \leas.ds.ky.gov\dfs\KYTCB00R01P\Active Projects\District05\RS&M\Bullitt 5-10035 Super replacement\5-10035\DETAILS\28807.dgn MicroStation v24.00.00.170

